

Influence of anchorage on flexural strength of beams strengthened with CFRP sheets

M. P. Ferreira^{1*} , M. H. Oliveira² , A. F. Lima Neto³ , L. S. Tapajós⁴ ,
A. J. C. Nascimento⁵ , M. C. Freire³ 

*Contact author: mpinaf@gmail.com

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ABSTRACT

In order to evaluate the influence of anchorage on the flexural strength of beams strengthened with Carbon Fiber Reinforced Polymer (CFRP) sheets, the experimental results of 126 tests in the literature and of an unprecedented series with 4 reinforced concrete beams, tested by these authors, were analyzed. The parameters affecting the performance and the strength of the beams are evaluated, and the design criteria of fib Bulletin 14 (2001) and ACI 440-2R (2008) are discussed. It was observed that, even with auxiliary devices in the PRFC anchorage, premature failure is possible, and that both theoretical recommendations lead to safe estimates, but excessively conservative in cases where the anchorage of the PRFC sheet is properly done.

Keywords: reinforced concrete; flexural strengthening; CFRP sheets; anchorage.

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¹ Civil Engineering Department, Federal University of Pará, Belém, Brazil.

² Civil and Environmental Engineering Department, University of Brasília, Brasília, Brazil.

³ Nucleus of Amazonian Development in Engineering, Federal University of Pará, Tucuruí, Brazil.

⁴ Civil Engineering Department, Federal University of the West of Pará, Itaituba, Brazil

⁵ Post-graduation in Civil Engineering, Federal University of Pará, Belém, Brazil.

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Influência da ancoragem na resistência à flexão de vigas reforçadas com mantas de PRFC

RESUMO

Buscando avaliar a influência da ancoragem na resistência à flexão de vigas reforçadas com mantas de Polímero Reforçado com Fibra de Carbono (PRFC), foram analisados os resultados experimentais de 126 ensaios presentes na literatura e de uma série inédita, ensaiada pelos autores, com 4 vigas de concreto armado. São avaliados os parâmetros que afetam o desempenho e a resistência das vigas, e discutidos os critérios de dimensionamento do fib Bulletin 14 (2001) e ACI 440-2R (2008). Observou-se que, mesmo com dispositivos auxiliares na ancoragem do PRFC, falhas prematuras são possíveis, e também que ambas as recomendações teóricas conduzem a estimativas a favor da segurança, porém excessivamente conservadoras nos casos onde a ancoragem da manta de PRFC é feita adequadamente.

Palavras-chave: concreto armado; reforço à flexão; mantas de PRFC; ancoragem.

Influencia del anclaje en la resistencia a la flexión de vigas reforzadas con mantas de PRFC

RESUMEN

Buscando evaluar la influencia del anclaje en la resistencia a la flexión de vigas reforzadas con mantas de Polímero Reforzado con Fibra de Carbono (PRFC), se analizaron los resultados experimentales de 126 ensayos presentes en la literatura y de una serie inédita de los autores, con 4 vigas de hormigón armado. Se evalúan los parámetros que afectan desempeño y resistencia de vigas, y se discuten los criterios de dimensionamiento del fib Bulletin 14 (2001) y ACI 440-2R (2008). Se observó que, incluso con dispositivos auxiliares en el anclaje del PRFC, fallos prematuros son posibles, y que ambas recomendaciones conducen a estimaciones seguras, pero excesivamente conservadoras en los casos en que el anclaje de la manta de PRFC se realice adecuadamente.

Palabras clave: hormigón armado; refuerzo de la flexión; mantas de PRFC; fondeadero.

NOMENCLATURE

a	– shear span	f_s'	– steel strength of the compressed reinforcement
b_f	– width of the layer of CFRP	f_{ys}	– yield strength of the tensile reinforcement
b_w	– width of the beam	f_{ys}'	– yield strength of the compressed reinforcement
c	– height of the block of rectangular compression equivalent of the concrete	h	– height of the beam
c_1	– factor obtained by calibration of results (equal to 0.64 for CFRP)	k_b	– geometric factor
d	– effective beam height	k_c	– constant that considers the compacting of the concrete during the concreting
d'	– centroid position of the compressed reinforcement	l_b	– anchoring length of the CFRP sheet
f_c	– concrete compression strength	l	– clamping width
f_{ctm}	– average tensile strength of concrete	n	– number of layers of CFRP
f_{fe}	– effective strength of the CFRP	t_f	– thickness of the CFRP
f_s	– strength in the steel of the tensile reinforcement	x	– position of the neutral axis

A_f	– area of reinforcement applied to the beam	T_s	– tensile component due to the tensile reinforcement
A_s	– steel area of the tensile reinforcement	α	– reduction coefficient due to the propagation of inclined cracks
A_s'	– steel area of compressed reinforcement	β_1	– coefficient that determines the approximation of the resultant compression curve of the concrete to a rectangle according to the recommendations of ACI 440-2R (2008)
C_C	– compression component due to the concrete portion	ϵ_{bi}	– strain found in the covering of the tensile reinforcement in the beam before the strengthen
C_S	– compression component due to compressed reinforcement	ϵ_c	– concrete strain
E_f	– modulus of elasticity of the CFRP	ϵ_{cu}	– concrete ultimate strain
E_s	– modulus of elasticity of steel	ϵ_f	– carbon fiber strain
F_{cc}	– compression resulting	ϵ_{fd}	– limit value of fiber strain to be adopted in the sizing and verification of reinforcement
L	– total beam length	ϵ_{fe}	– effective strain in the CFRP
M	– moment applied during the experimental test	ϵ_{fu}	– ultimate strain observed in the polymer at the moment of failure
M_R	– resistant moment in the cross section of the beam	ϵ_s	– strain in the tensile reinforcement
M_{Rteo}	– resistant moment estimated by the recommendations	ϵ_s'	– strain in the compressed reinforcement
M_u	– experimental ultimate moment	ϵ_{ys}	– yield strain of the flexural reinforcement
M_{ACI}	– ultimate moment according to the recommendations of ACI 440-2R (2008)	r	– rate of the reinforcement of the beam before the strengthen
M_{fib}	– ultimate moment according to the recommendations of <i>fib</i> Bulletin 14 (2001)	rr	– beam reinforcement ratio after strengthen
M_{V1}	– experimental ultimate moment of the beam V1	\emptyset	– reinforcement bar diameter
$N_{fa,max}$	– maximum force that can be applied to the reinforcement of the beam	ψ	– coefficient that determines the approximation of the resulting curve of concrete compression to a rectangle, according to the recommendations of <i>fib</i> Bulletin 14 (2001)
P	– applied load in the experimental test		
R^2	– coefficient of correlation of the results		
T_f	– tensile component due to the fiber reinforcement portion		

1. INTRODUCTION

Among the techniques used for the strengthening of reinforced concrete structures, we highlight the systems that use Fiber Reinforced Polymers (FRP), for presenting low weight and high tensile and corrosion strength, and the Carbon Fiber Reinforced Polymers (CFRP) that have shown significant acceptance for more than two decades due to their high values of strength, stiffness and durability, as well as the ease of installation when compared to other types of fibers (Monti and Liotta, 2007). In the specific case of flexural reinforcement of reinforced concrete beams, several researchers show that the use of sheets of CFRP is an effective methodology, highlighting the works of Rafi et al. (2008), Khan and Fareed (2014) and Hawileh et al. (2015).

However, it is important to feature that premature failures are associated with its use. In the case of flexural reinforced beams, this brittle fracture may occur due to the detachment of the carbon fiber from the concrete substrate, which may limit the increase in strength provided by the reinforcement. This mode of brittle ruin is usually associated with deficiency in the anchorage of the reinforcement system, which makes the use of additional devices recommended, such as band clipping of CFRP sheet (see Benjeddou *et al.*, 2007; Dong *et al.*, 2011, Kim and Shin, 2011, among others).

This work presents a series of experimental tests on reinforced concrete beams strengthened with CFRP sheets, carried out with the objective of evaluating the influence of the anchorage in the reinforcement structural performance. In addition, a large database was set up, with experimental results selected from the work of different authors. This database is used to discuss the influence of different parameters on the performance of reinforcement with CFRP sheets and to evaluate the performance of theoretical predictions of strength obtained using the recommendations presented by ACI 440-2R (2008) and fib Bulletin 14 (2001).

2. CFRP STRENGTHENED BEAMS

2.1 Failure modes

Teng *et al.* (2003) state that beams strengthened to flexure with CFRP sheets may exhibit brittle failure modes, for instance: by exhaustion of the strength of the CFRP (see Figure 1a); detachment of the reinforcement at the interface with the concrete (see Figure 1b); and separation of the reinforcement together with the concrete covering of the flexural reinforcement (see Figure 3c). The failure of the CFRP sheet can occur in weakly reinforced beams by flexure, being a brittle mode of failure since the CFRP presents linear-elastic behavior until the failure. The detachment of the sheet may occur due to lack of the anchorage of reinforcement, excessive cracking of the beam or, faults in its bonding process. The separation of concrete cover layer can be caused by shear strengths at the interface between concrete and CFRP due to the difference between the moduli of elasticity of these materials, which can be amplified by the corrosion of the bending reinforcement.

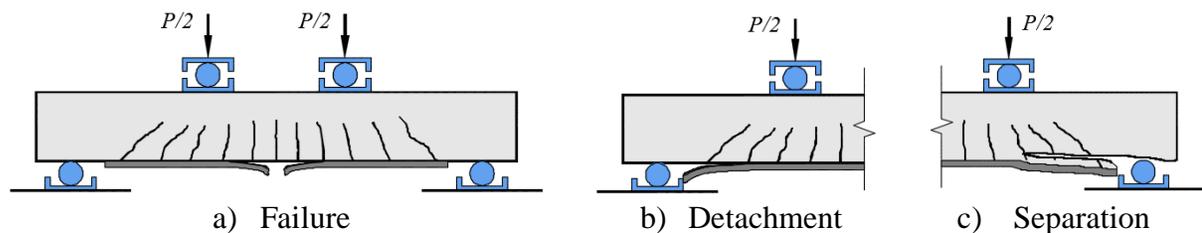


Figure 1. Failure modes of beams strengthened to flexure with CFRP.

2.2 Flexural Strength

The behavior of a reinforced concrete beam strengthened with CFRP and subjected to bending can be expressed according to the diagram of Figure 2. In this analysis, it's considered that the ratio of the reinforced beam framework is equivalent to the sum of the initial reinforcement rate with contribution from the reinforcement, as presented in (1).

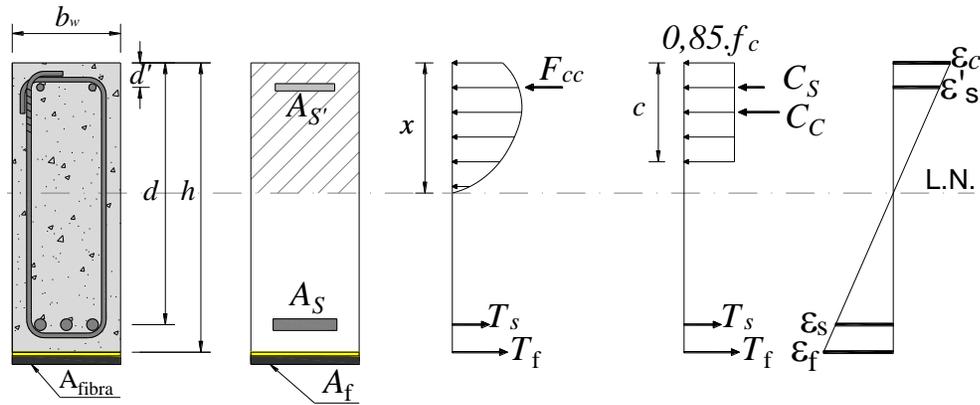


Figure 2. Stress-strain diagram of a beam strengthened with CFRP.

$$\rho_r = \rho + \frac{A_f \cdot E_f}{b_w \cdot h \cdot E_s} \quad (1)$$

Where: ρ is the rate of the reinforcement of the beam before the strengthening; A_f is the area of reinforcement applied to the beam; E_f is the modulus of elasticity of the CFRP; E_s is the modulus of elasticity of steel; b_w is the width of the beam; h is the height of the beam.

2.2.1 ACI 440-2R (2008)

The American code ACI 440-2R (2008) presents recommendations for the sizing of reinforcement using CFRP. To determine the flexural strength of reinforced concrete beams reinforced with carbon fiber sheets, (2) - (10) are used.

$$\epsilon_{fd} = 0,41 \cdot \sqrt{\frac{f_c}{n \cdot E_f \cdot t_f}} < 0,9 \cdot \epsilon_{fu} \quad (2)$$

$$\epsilon_{fe} = \epsilon_{cu} \cdot \left(\frac{h-x}{x} \right) - \epsilon_{bi} \leq \epsilon_{fd} \quad (3)$$

$$f_{fe} = E_f \cdot \epsilon_{fe} \quad (4)$$

$$\epsilon_s' = \epsilon_{cu} \cdot \left(\frac{x-d'}{x} \right) \quad (5)$$

$$\epsilon_s = \epsilon_{fe} \cdot \left(\frac{d-x}{h-x} \right) \quad (6)$$

$$f_s' = E_s \cdot \epsilon_s' \leq f_{ys}' \quad (7)$$

$$f_s = E_s \cdot \epsilon_s \leq f_{ys} \quad (8)$$

$$x = \frac{(A_s \cdot f_s) + (A_f \cdot f_{fe}) - (A_{s'} \cdot f_s')}{\beta_1 \cdot 0,85 \cdot f_c \cdot b_w} \quad (9)$$

$$M_R = A_s \cdot f_s \cdot \left(d - \frac{\beta_1 \cdot x}{2} \right) + 0,85 \cdot A_f \cdot f_{fe} \cdot \left(h - \frac{\beta_1 \cdot x}{2} \right) + A_s' \cdot f_s' \cdot \left(\frac{\beta_1 \cdot x}{2} - d' \right) \quad (10)$$

Where: ε_{fd} is the limit value of fiber strain to be adopted in the sizing and verification of reinforcement to avoid premature fiber failure; n is the number of layers of CFRP; t_f is the thickness of the CFRP; ε_{fu} is the ultimate strain observed in the polymer at the moment of failure; ε_{fe} is the effective strain in the CFRP; x is the position of the neutral axis; ε_{bi} is the strain found in the covering of the tensile reinforcement in the beam before the strengthen; f_{fe} is the effective strength of the CFRP; d is the effective beam height; ε_s' is the strain in the compressed reinforcement; f_s' is the strength in the steel of the compressed reinforcement; d' is the centroid position of the compressed reinforcement; ε_s is the strain in the tensile reinforcement; f_s is the strength in the steel of the tensile reinforcement; A_s is the steel area of the tensile reinforcement; A_s' is the steel area of compressed reinforcement; β_1 is a coefficient that determines the approximation of the resultant compression curve of the concrete to a rectangle, being 0.85 for concrete with f_c values lower than 28 MPa and there being a linear decrease of 0.05 for each 7 MPa above that limit of strength, the minimum value for such a coefficient, according to ACI 318 (2014), is 0.65; M_R is the tough moment in the cross section of the beam.

2.2.2 *fib* Bulletin 14 (2001)

The *fib* Bulletin 14 (2001) presents recommendations for the dimensioning of flexural reinforced beams with CFRP and adopts a calculation philosophy like that adopted by ACI 440-2R (2008). The same equations presented by the ACI are used in this document, differentiating only the following parameters: the coefficient β_1 in it is called of ψ and equals to 0.8, independent of the class of strength of the concrete; and the strain limit of the fiber, which is calculated by (11), whose parameters are found by means of (12) - (14).

$$\varepsilon_{fd} = \frac{N_{fa,max}}{E_f \cdot A_f} \quad (11)$$

$$N_{fa,max} = \alpha \cdot c_1 \cdot k_c \cdot k_b \cdot b_w \cdot \sqrt{E_f \cdot t_f \cdot f_{ctm}} \quad (12)$$

$$k_b = 1,06 \sqrt{\frac{2 - \frac{b_f}{b_w}}{1 + \frac{b_f}{400}}} \geq 1 \quad (13)$$

$$f_{ctm} = 0,33 \cdot \sqrt{f_c} \quad (14)$$

Where: $N_{fa,max}$ is the maximum force that can be applied to the reinforcement of the beam (expressed in N); α is a reduction coefficient due to the propagation of inclined cracks, adopted as 0.9; c_1 is equal to 0.64; k_c is a constant that considers the compacting of the concrete during the concreting, being this value equal to 1 when the strengthening is applied in the underside of the beam and 0.67 in the upper face; k_b is a geometric factor; f_{ctm} is the average tensile strength of concrete, as expressed in Eurocode 2 (2004); b_f is the width of the layer of CFRP.

3. EXPERIMENTAL PROGRAM

3.1 Characteristics of the beams

Tests were carried out on 4 reinforced concrete beams strengthened with CFRP sheets, having as variables the sheet anchorage length and width in order to evaluate their influence on both the performance and the strength of the beams. It was tested 1 reference beam without reinforcement and 3 reinforced beams with carbon fiber sheet. The rates of steel reinforcement and CFRP were kept constant. Table 1 and Figure 3 show the main characteristics of the beams. The strains in the flexural reinforcement, the reinforcement sheet and the clamping were monitored with electrical extensometers, as shown in Figure 4. Figure 5 shows the test system of the beams.

Table 1. Characteristics of the tested beams.

Beam	l (mm)	l_b (mm)	b_f (mm)	A_f (mm ²)	t_f (mm)	E_f (GPa)	ϵ_{fu} (%)
V1	-	-	-	-	-	-	-
V2	100	285	120	19.92	0.166	230	2.1
V3		385					
V4	150						

$b_w = 120$ mm; $h = 200$ mm; $d = 166$ mm; $A_s = 101$ mm²; $f_c = 20$ MPa; $a = 400$ mm

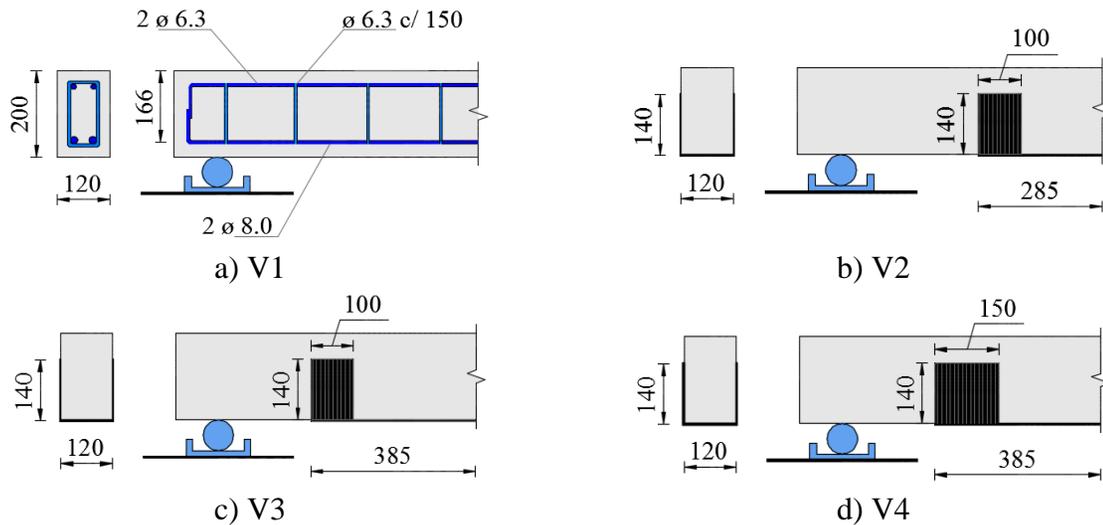


Figure 3. Characteristics of tested beams.



Figure 4. Monitoring of beams.

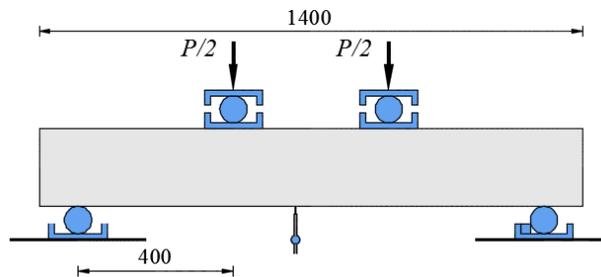


Figure 5. Test system of beams.

3.2 Discussion of Results

The failure mode of each of the beams tested can be visualized in Figure 6. It was observed that the reference beam failed by flexure after its longitudinal reinforcement reached high levels of strain (Figure 6a). In the V2 beam the concrete was pulled out in the zone adjacent to the clamping (Figure 6b). The V3 beam lost its strength after the fiber detachment occurred in part of the clamping contact zone (Figure 6c). However, the V4 beam failed after the detachment of the concrete covering in the region of the flexure span (Figure 6d).

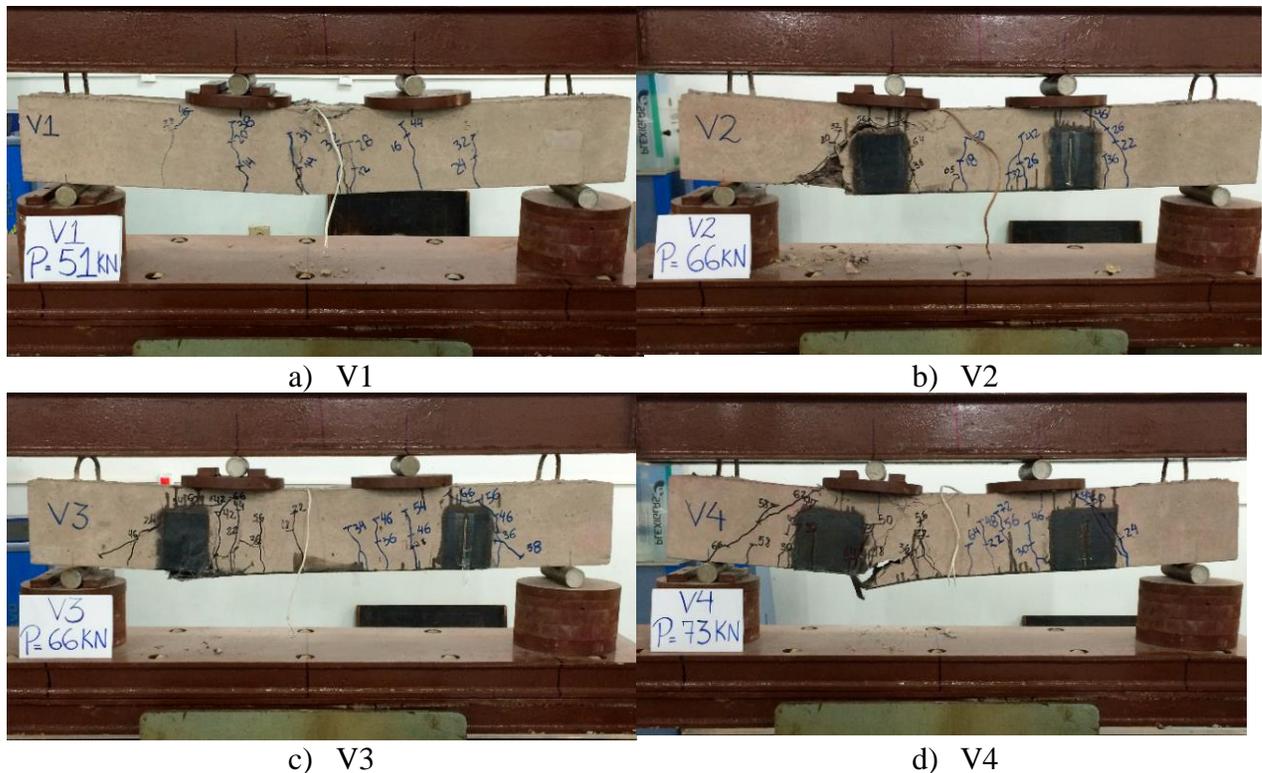


Figure 6. Failure surface of tested beams.

Figure 7 shows the results of strains measured in the flexural reinforcement and at different points of the PRFC. It is noticed that, in all the beams, the flexural reinforcement reached the yielding, as shown in Figure 7a. The strains in the flexural reinforcement were smaller in the beams with the sheet of CFRP for the same load levels, since the reinforcement contributed in the tensile part of the piece. In addition, it was observed that the resistant behavior of the beams with the sheet was more brittle than that of the non-reinforced beam, since after reaching the maximum strength, the readings were interrupted by the failure of the part, whereas in the beam V1, if the maximum load was reached, this loading level remained associated to a high level of strain. Figure 7b shows that

both the anchoring length of the fiber and the clamping width influenced the ultimate strain of the CFRP measured in the tests, varying from 3.5 ‰ for the beam V2 to approximately 4.2 ‰ for the beams V3 and V4. In all cases, it should be noted that these values are higher than theoretically predicted by the ACI, which would be 1.9 ‰. It should be emphasized that none of the beams failed with the exhaustion of the tensile strength of the sheet of CFRP. Figure 7c shows that in all the beams the level of strain at the end of the reinforcement was smaller than that measured in the middle of the span, where the moment is maximum and that in the case of the beam V2, which failed due to the detachment of the fiber in this region, the strain limit measured was practically the same as that suggested by the manufacturer of 2.1 ‰. In Figure 7d it is possible to notice that the level of strain developed in the region of the clamping was small and that in beam V3 it clearly begins to separate before the failure.

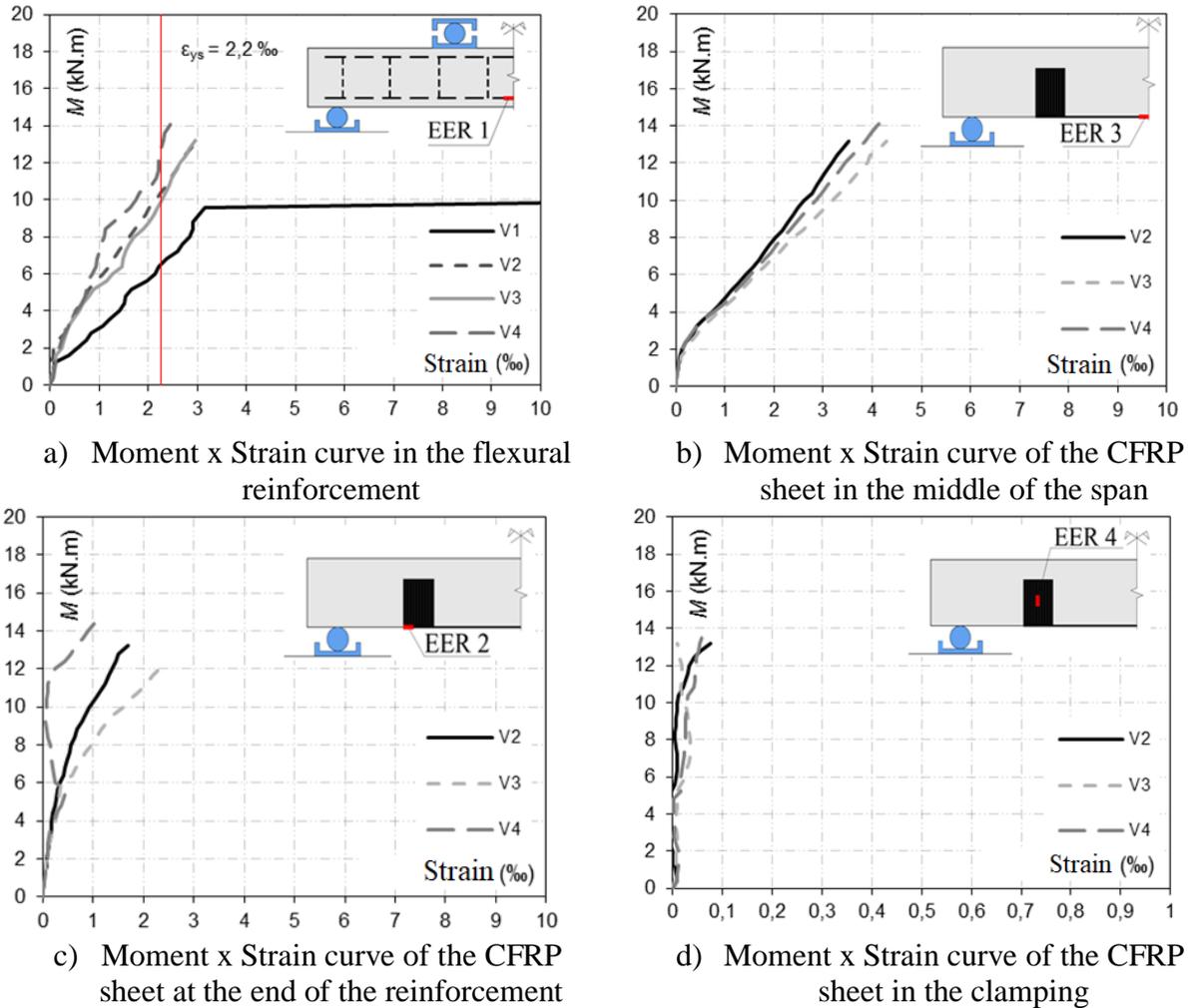


Figure 7. Results of the beams tests.

Figure 8 shows the vertical displacement curve of the beams, measured in the middle of the span. It is possible to notice that the reinforced beams presented a very similar response, showing greater rigidity than the reference beam, which failed by flexure in a ductile manner and showed large displacements when reaching the moment of yield of its flexural reinforcement. As seen, all the reinforced beams failed abruptly, with failure occurring in different regions of the CFRP.

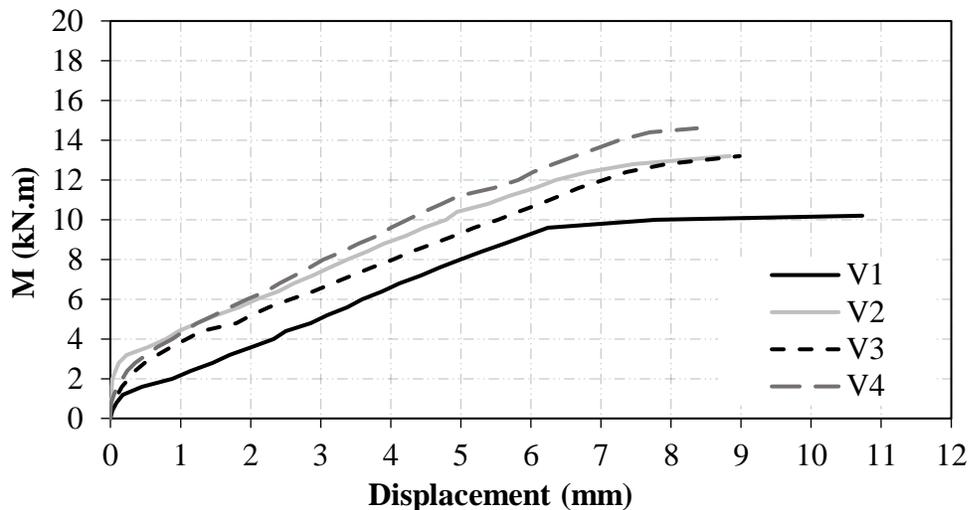


Figure 8. Moment-displacement curves of the tested beams.

Table 2 presents the final loads of the tested beams and compares the strength of the reinforced beams with the sheet of CFRP with the reference beam (V1), in order to determine the increment of strength generated by the reinforcement. In addition, the experimental strength of the beams is compared with theoretical estimates obtained following the recommendations of the ACI and *fib*14. From these results, it was observed that the increase of the anchoring length of the beam V3 in relation to the beam V2 did not result in an increase of strength, and that the increase of the clamping width was responsible for the greater strength of the beam V4 in relation to the beam V3. It was also observed that the use of ACI would result in predictions of strength against safety for beams V2 and V3, while *fib*14 presented results in favor of safety.

Table 2. Ultimate loads of the beams.

Beam	M_u (kN.m)	M_u/M_{V1}	M_u/M_{ACI}	M_u/M_{fib}
V1	10.2	1.00	-	-
V2	13.2	1.29	0.94	1.14
V3	13.2	1.29	0.94	1.14
V4	14.6	1.43	1.04	1.26

4. DATABASE

4.1 Methodology of collection and analysis of data

In addition to the experimental program, a database was collected with the results of 126 tests of 20 authors, involving flexural reinforced beams with the sheet of CFRP. As a criterion for the choice of beams, only those that failed by bending, without initial loading and externally bonded reinforcement, were selected. The objective of this data analysis is to evaluate the influence of the main variables on the flexural strength of reinforced beams with CFRP.

The analysis of the database involves the comparison of the experimental results with the theoretical estimates obtained using the ACI and *fib* 14. Thus, it was calculated the values of average, coefficient of variation and standard deviation of the ratio between the experimental ultimate moment and the theoretical resistant moment (M_u/M_{Rteo}), and finally, the percentage of safety results ($M_u/M_{Rteo} < 1$) was evaluated. In order to evaluate the precision of the theoretical models, graphs are presented that compare the moment of experimental failure (M_u) by the function of the predicted resistant moment (M_{Rteo}), in order to analyze if the tendency of the obtained results approaches the ideal condition ($M_u = M_{Rteo}$).

As previously noted, the performance of the reinforcement with sheet of CFRP is directly related to the quality of fiber anchoring to concrete. Therefore, the beams of the database were divided into 3 groups: group 1 for situations where the bonding of the sheet stretched from the center of the beam to the support or beyond; the group 2 with beams in which the glue of the sheet did not extend to the support; and the group 3 with beams in which external devices were used to aid in reinforcement anchorage, such as screws or restrainers. It should be noted that this type of anchorage is considered by several authors as the most favorable for anchoring the CFRP sheet, since that the device is made of this same material.

Another method used was the Collins (2001) criterion, known as *Demerit Points Classification* (DPC), in which the values of M_u/M_{Rteo} were classified in ranges from "extremely dangerous" to "extremely conservative", with performance of the theoretical model defined as a number resulting from the sum of the products from M_u/M_{Rteo} by their corresponding score, according to the classification. Table 3 presents a summary of the characteristics of the beams of the database, as well as the symbology used by the authors to identify the figures. Table 4 shows a summary of the variables related to the reinforcement with carbon fiber sheet of the beams of the database. Table 5 presents the parameters related to Collins DPC (2001).

Finally, were prepared graphs which relate the experimental strength of the beams to the predicted theoretical failure (M_u/M_{Rteo}) according to the number of layers of CFRP sheets, as well as the reinforcement ratio after strengthening in relation to the initial reinforcement (ρ_f/ρ). The purpose of these charts is to analyze if the hypotheses adopted by the theoretical models present adequate correlation with the existing experimental evidence.

Table 3. Characteristics of the database beams.

Author	No. of beams	b_w (mm)	h (mm)	d (mm)	a (mm)	A_s (mm ²)	f_{ys} (MPa)	f_c (MPa)
Beber (2003)	12	150	300	272	833	245	706	32
Beber <i>et al.</i> (2000)	8	120	250	219	783	157	565	33
David <i>et al.</i> (2003)	4	150	300	267	933	307	500	39
Esfahani <i>et al.</i> (2007)	6	150	200	164	600	402-626	350-406	24
Breña <i>et al.</i> (2003)	9	203	356-406	318-368	1065-1220	395	440	35-37
Rusinowski <i>et al.</i> (2009)	5	200	300	262	1300	402	527	64-70
Toutanji <i>et al.</i> (2006)	7	108	158	127	560	142	427	49
Barros <i>et al.</i> (2007)	6	120	170	141-145	300	39-99	627-788	44
Gamino (2007)	14	75	150	120	550	62	640	45
Zhang <i>et al.</i> (2006)	4	120	250	224	750	226-402	335	23
Spadea <i>et al.</i> (2000)	2	140	300	266	1800	402	435	30
Alagusundaramoorthy <i>et al.</i> (2003)	12	230	380	342	1830	981.75	414	31
Ferrari (2007)	3	170	350	300	950	254.4	548	35-38
Dias <i>et al.</i> (2002)	5	120	180	160	720	100.5	533	41
Balaguru e Kurtz (2001)	3	200	300	255	1000	258	447	47
Vieira <i>et al.</i> (2016)	8	120	245	220	800	157-245	500	44
Bilotta <i>et al.</i> (2015)	2	120	160	135	925	157	590	21
Garcez (2007)	2	150	300	270	950	245.4	578	41.4
Juvandes (1999)	9	75-150	150	130	605-650	14-226	192-507	20-45
Chahrour e Soudki (2005)	5	150	250	219	750	402.1	400	39

Table 4. Strengthening variables of the beams.

Author	No. of beams	No. of layers	b_f (mm)	t_f (mm)	A_f (mm ²)	E_f (GPa)	e_{fu} (‰)
Beber (2003)	12	1-6	50-150	0.1-1.4	10-140	240	12-14
Beber <i>et al.</i> (2000)	8	1-10	120	0.011	1.3-13.2	230	15
David <i>et al.</i> (2003)	4	2-4	50	1.2	120-240	150	15
Esfahani <i>et al.</i> (2007)	6	1-2	100-150	0.176	17.6-52.8	237	12
Breña <i>et al.</i> (2003)	9	1-2	50-100	0.165-1.2	16.5-104	62-230	12-16
Rusinowski <i>et al.</i> (2009)	5	1-2	50-120	1.4	140-168	155-300	9-15
Toutanji <i>et al.</i> (2006)	7	3-6	102	0.165	50.5-101	110	6
Barros <i>et al.</i> (2007)	6	1-3	9.6-80	0.1-1.4	13.4-40.3	158.8-240	15-17
Gamino (2007)	14	1-2	75	0.11-0.13	8.2-16.5	230-235	15
Zhang <i>et al.</i> (2006)	4	1-2	120	0.11	13.3-26.6	235	14.27
Spadea <i>et al.</i> (2000)	2	1	80	1.2	96	152	15.1
Alagusundaramoorthy <i>et al.</i> (2003)	12	1-3	76-203	1.4-4.7	36.4-975	48-228	11.5-15
Ferrari (2007)	3	1-3	16.5	0.17	2.8-8.4	50	13
Dias <i>et al.</i> (2002)	5	1-2	20-70	0.1-1.4	15.5-28	200-240	11-15
Balaguru e Kurtz (2001)	3	2-5	152	0.071	21.6-54	200	6
Vieira <i>et al.</i> (2016)	8	2-5	100	0.166	33.2-83	230	21
Bilotta <i>et al.</i> (2015)	2	1-2	40	1.4	56-112	171	12
Garcez (2007)	2	1-2	150	0.165	247-49	227	15
Juvandes (1999)	9	1	50	1.2	60	155	19
Chahrour e Soudki (2005)	5	1	100	1.2	120	155	19

Table 5. Collins criterion.

M_{exp}/M_{Rteo}	Classification	Penalty
< 0.50	Extremely Dangerous	10
(0.50 - 0.65)	Dangerous	5
(0.65 - 0.85)	Low Safety	2
(0.85 - 1.15)	Proper Safety	0
(1.15 - 2.00)	Conservative	1
≥ 2.00	Extremely Conservative	2

4.2 Discussion of Results

Figure 9 shows the graphs that confront the experimental strength of the database with the theoretical predictions, including in red, the experimental results tests of this research. It was observed that the two theoretical models presented similar performance in relation to the average of the results, but with *fib* 14 showing greater dispersion and higher percentage of results against safety. Moreover, the tested beams fit well in the trend of results observed for the larger universe generated by the database.

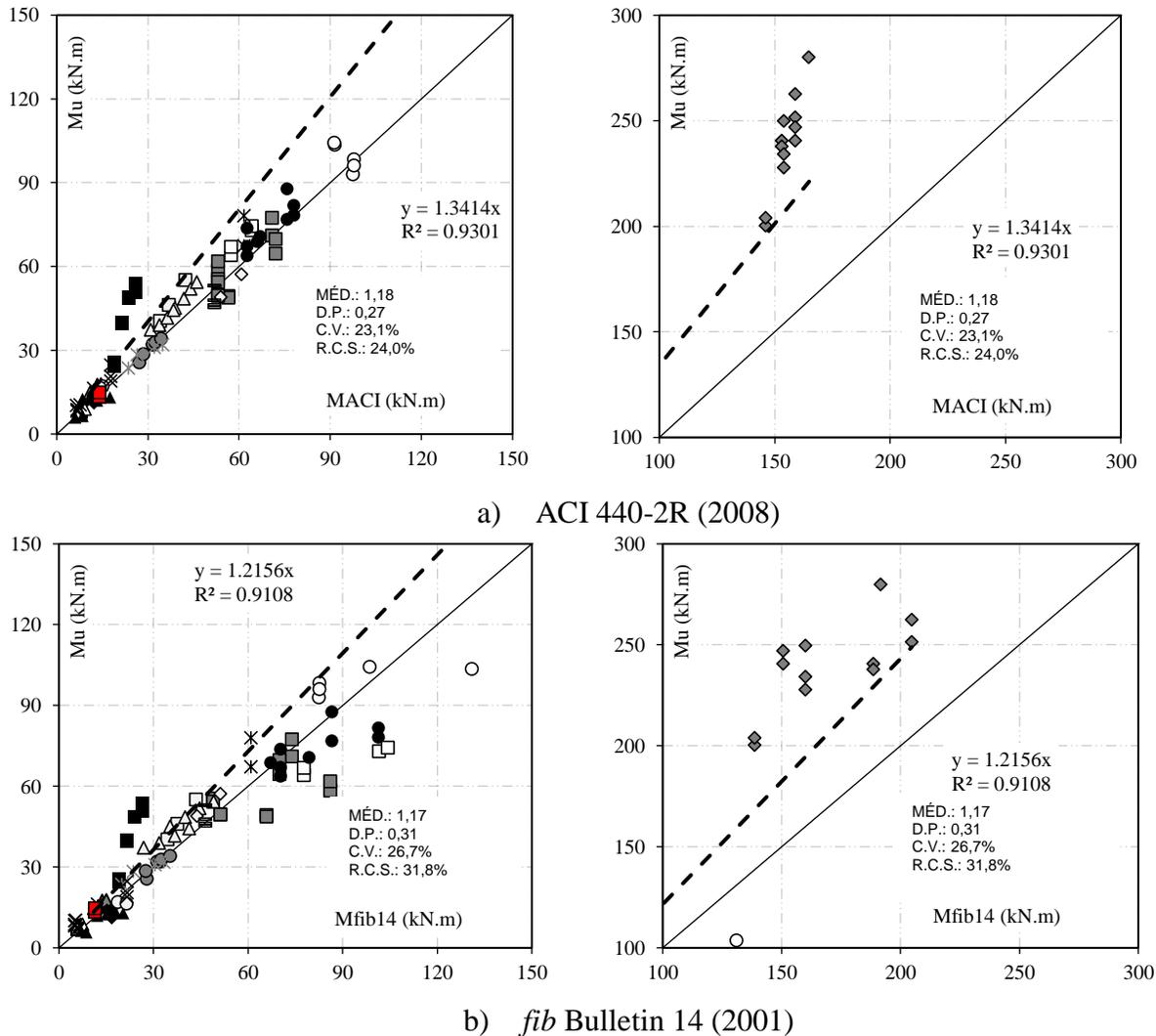


Figure 9. Analysis of the theoretical model's accuracy.

In order to evaluate the influence of the type of anchorage of the reinforcement with CFRP in the performance of the theoretical predictions, Figure 10 divides the general data presented in Figure 9, classifying them by type of anchorage in 3 distinct groups. For group 1, ACI presented an average closer to 1.0, as well as a lower dispersion of results and approximately the same number of values against safety, when compared with *fib* 14. In group 2, this behavior was reversed, and *fib* 14 showed lower average, but still with greater dispersion and greater percentage of results against the safety. For the beams of group 3 the theoretical methods presented similar performance, with the ACI showing again less dispersion.

As seen, using Collins criterion (2001), it is possible to establish scores for the two recommendations, based on the sample space of the collected database, as shown in Figure 11. It is observed that the ACI presents a lower penalty according to the criterion adopted.

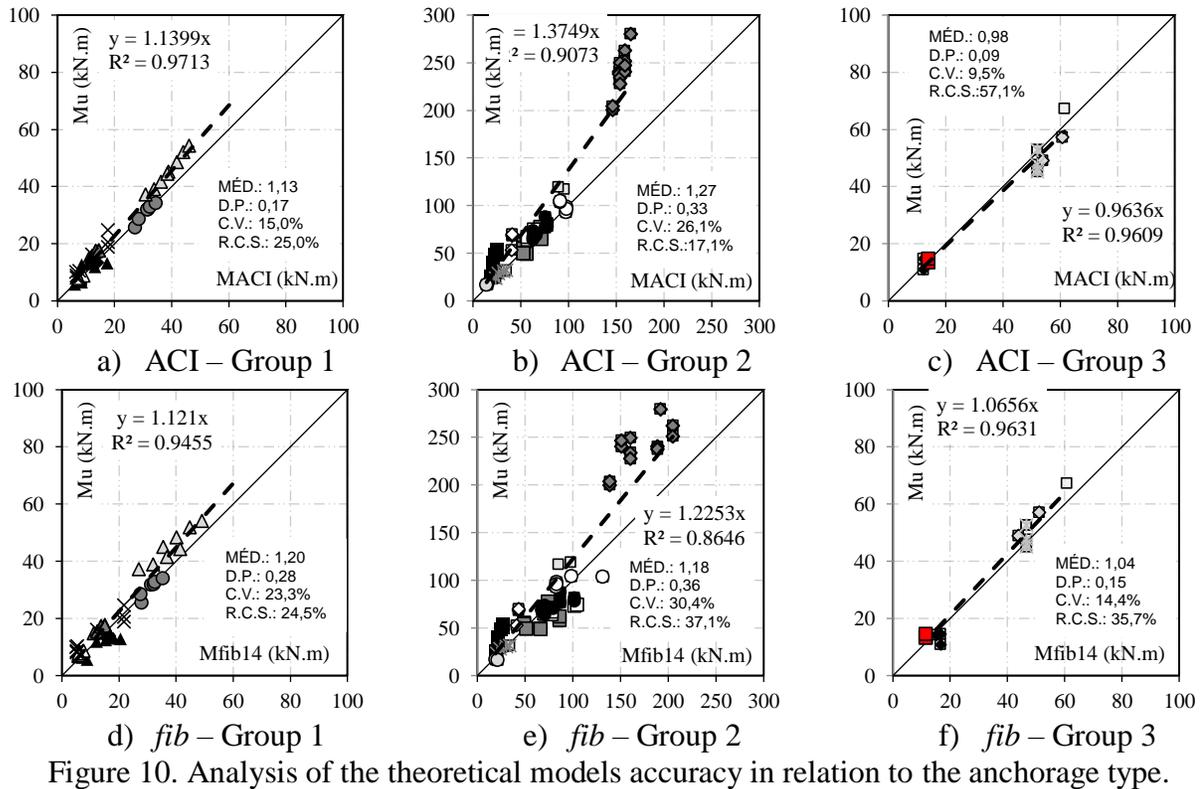


Figure 10. Analysis of the theoretical models accuracy in relation to the anchorage type.

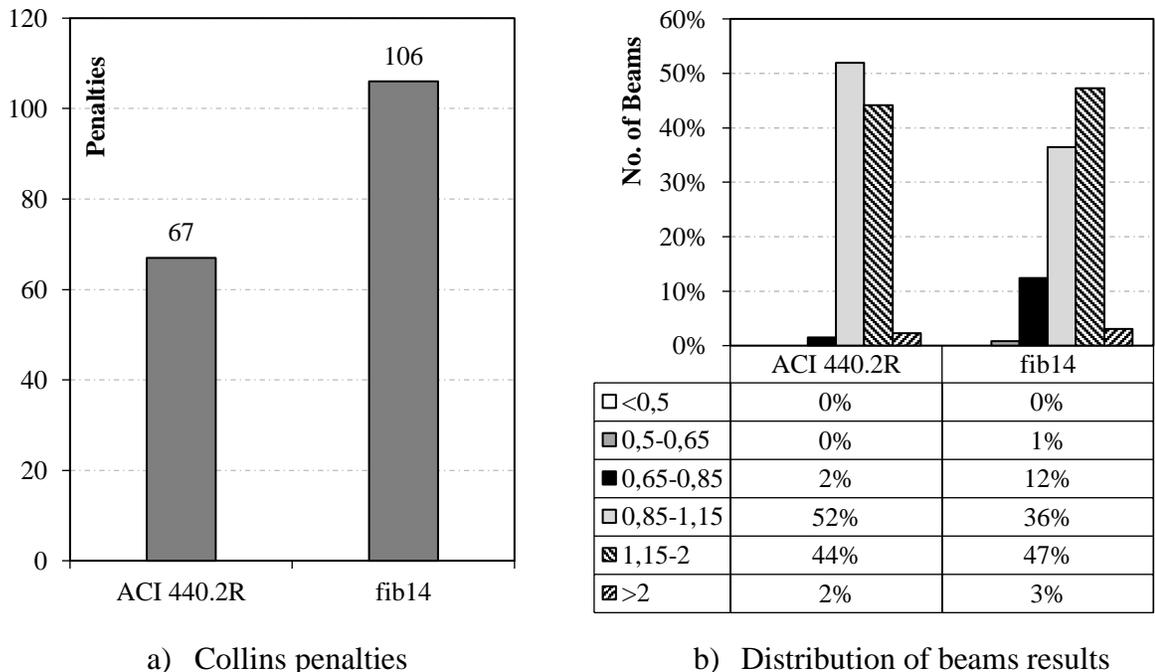


Figure 11. Result of the Collins criterion (2001).

Figure 12 analyzes the influence of the number of layers of CFRP on both reinforcement performance and theoretical methods. It is noticed that there is a trend of more conservative results as the amount of layers increases, which is justified by the fact that both recommendations limit the strain of CFRP. For the ACI, this trend is slightly higher, since in its equation the number of layers is used, which penalizes the strength estimation of reinforced beams with a high number of layers. In contrast, in *fib* 14 this factor is not considered.

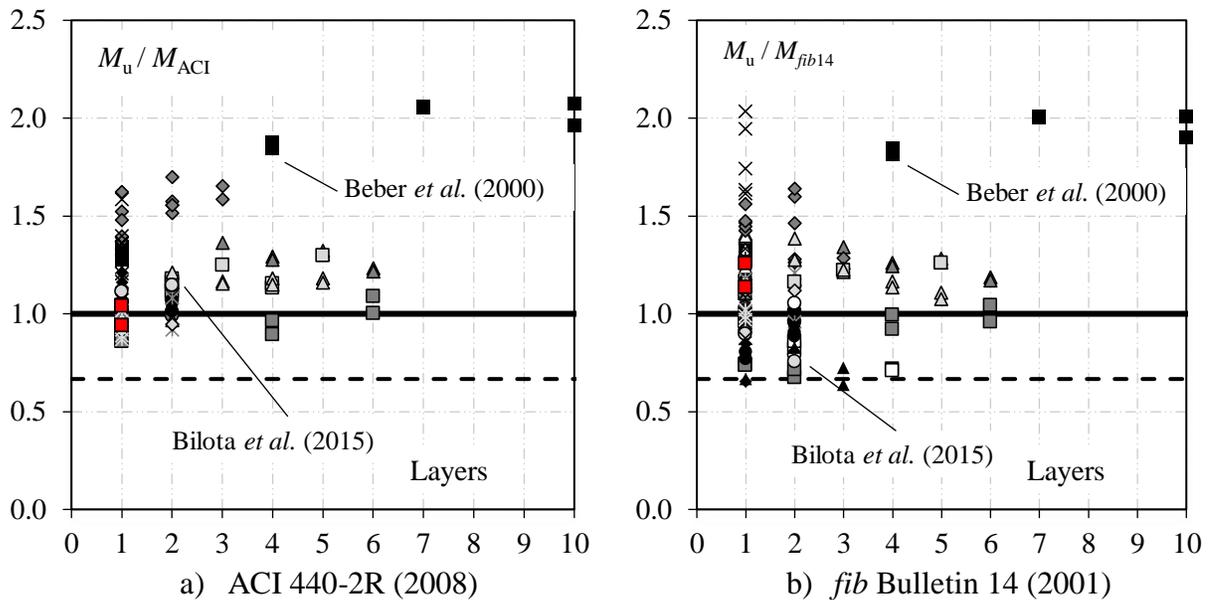


Figure 12. Influence of the number of strengthening layers.

Figure 13 shows the influence of the proportion between the reinforcement ratio after the strengthening and the flexural reinforcement ratio (ρ_r/ρ). There is a trend of less conservative results as ρ_r/ρ increases, and for values above 1.4, ACI tends to stabilize this trend, avoiding estimates of strength against safety. However, the same was not observed for *fib* 14, whose predictions approached the dashed line, which expresses the limit of the design strength.

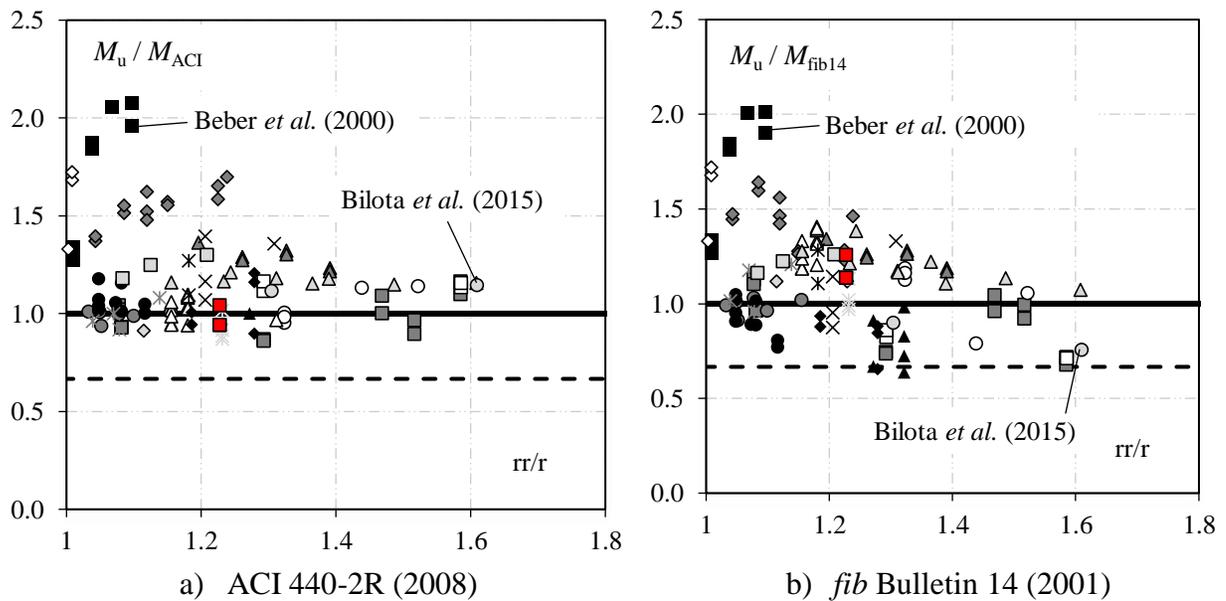


Figure 13. Influence of the reinforcement ratio increase (ρ_r/ρ)

Figures 12 and 13 show that the number of layers of CFRP is not necessarily a parameter that should be penalized in the prediction of strength of flexural reinforced beams with sheets of CFRP as defined by the ACI. This is evident from, for example, the results of Beber *et al.* (2000), who tested beams with up to ten layers of CFRP sheets and obtained good performance, and those of Bilota *et al.* (2015), whose beams tested had only one or two layers of CFRP and showed much lower performance. By analyzing Figure 13, the beams of Beber *et al.* (2000) present low values of ρ_r/ρ while those of Bilota *et al.* (2015) presented extreme values within the beams of this

database, evidencing that ρ_r/ρ is a fundamental parameter to be taken into consideration in the design.

5. CONCLUSION

In order to observe the influence of reinforcement anchorage with CFRP on the behavior of beams resisting to flexure, an experimental study was carried out, involving tests on 4 reinforced concrete beams, varying the anchorage criteria of the carbon fiber sheets. In addition to the experimental approach, a database with results from several authors was used to evaluate the influence of different parameters on the strength of reinforced beams to flexion with CFRP sheets. These data were also used to discuss the performance of the theoretical methods proposed by *fib* Bulletin 14 (2001) and ACI 440-2R (2008).

As for beam tests, it was observed that the clamping width had a greater influence on the flexural strength of the reinforced beams with CFRP sheet, since the V4 beam presented the greatest strength in relation to the others, even though the anchoring length was identical to the one of the beam V3. The beams V2 and V3 presented the same values of strength, although the beam V3 presented a greater length of anchorage between them. Another important point is that, even with the strength additions of the reinforced beams in relation to the reference, the beams with CFRP sheet have failed prematurely. Finally, it was observed that the limit values of strain of the CFRP sheet assumed by the manufacturer are very conservative in face of the experimental results recorded in this research.

As for the evaluation of the theoretical models, it is verified that both the *fib* 14 and the ACI presented conservative results in relation to the M_u/M_{Rteo} ratio, which in practice should guarantee predictions of strength in favor of safety in most cases. It was also observed that the procedure of reducing reinforcement efficiency by means of limitations on CFRP strains, although generally in favor of safety, may become excessively conservative in cases of reinforcement where CFRP anchorage is performed properly. Finally, it is emphasized that among the evaluated parameters that affect the flexural strength of beams with CFRP sheets, it is worth noting that the ratio ρ_r/ρ was more relevant than the number of layers of CFRP sheets, considering the data of this database.

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